

# **Collapse risk assessment of a Chilean dual wall-frame reinforced concrete office building**

G. Araya-Letelier

P.F. Parra

D. Lopez-Garcia

A. Garcia-Valdes

G. Candia

R. Lagos

**a**Escuela de Construcción Civil, Pontificia Universidad Católica de Chile, Avenida Vicuña Mackenna 4860, Macul, Santiago 7820436, Chile

**b**Facultad de Ingeniería y Ciencias, Universidad Adolfo Ibáñez, Diagonal Las Torres 2640, Peñalolén, Santiago 7941169, Chile

**c**Department of Structural & Geotechnical Engineering, Pontificia Universidad Católica de Chile, Avenida Vicuña Mackenna 4860, Macul, Santiago 7820436, Chile

**d**Facultad de Ingeniería Civil, Universidad del Desarrollo, Avenida Plaza 680, Las Condes, Santiago 7610658, Chile

**e**National Research Center for Integrated Natural Disaster Management (CIGIDEN), CONICYT FONDAP 15110017, Avenida Vicuña Mackenna 4860, Macul, Santiago 7820436, Chile

**f**Rene Lagos Engineers, Magdalena 140, Las Condes, Santiago 7550104, Chile

**Keywords:** Collapse assessment, Chilean code-conforming building, Incremental dynamic analyses, Seismic hazard models

## **ABSTRACT:**

Several code-conforming reinforced concrete buildings were severely damaged during the 2010 moment magnitude ( $M_w$ ) 8.8 Chile earthquake, raising concerns about their real collapse margin. Although critical updates were introduced into the Chilean design codes after 2010, guidelines for collapse risk assessment of Chilean buildings remain insufficient. This study evaluates the collapse potential of a typical dual system (shear walls and moment frames) office building in Santiago. Collapse fragility functions were obtained through incremental dynamic analyses using a state-of-the-art finite element model of the building. Site-specific seismic hazard curves were developed, which explicitly incorporated epistemic uncertainty, and combined with the collapse fragility functions to estimate the mean annual frequency of collapse ( $\lambda_c$ ) values and probabilities of collapse in 50-years ( $P_c(50)$ ). Computed values of  $\lambda_c$  and  $P_c(50)$  were on the order of  $10^{-5}$ – $10^{-4}$ , and 0.1–0.7%, respectively, consistent with similar studies developed for buildings in the US. The results also showed that the deaggregation of  $\lambda_c$  was controlled by small to medium earthquake intensities and that different models of the collapse fragility functions and hazard curves had a non-negligible effect on  $\lambda_c$  and  $P_c(50)$ , and thus, propagation of uncertainty in risk assessment problems must be adequately taken into account.